

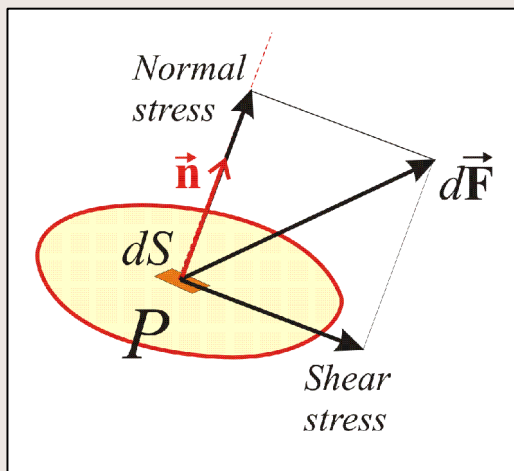
# Elasticity and Seismic Waves

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- Macroscopic theory
- Rock as 'elastic continuum'
  - ◆ Elastic body is deformed in response to stress
  - ◆ Two types of deformation: Change in **volume** and **shape**
- Equations of motion
- Wave equations
- Plane and spherical waves
  
- **Reading:**
  - › Shearer, Sections 2, 3
  - › Telford *et al.*, Section 4.2.

# Stress

- Consider the interior of a deformed body:



At point  $P$ , force  $d\mathbf{F}$  acts on any infinitesimal area  $dS$

Stress, with respect to direction  $\mathbf{n}$ , is a vector:

$$\lim(d\mathbf{F}/dS) \text{ (as } dS \rightarrow 0)$$

- Stress is measured in [*Newton/m<sup>2</sup>*], or *Pascal*
  - ◆ Note that this is a unit of pressure
- $d\mathbf{F}$  can be decomposed in two components relative to  $\mathbf{n}$ :
  - ◆ Parallel (*normal stress*)
  - ◆ Tangential (*shear stress*)

# Stress

- Stress, in general, is a *tensor*:
  - ◆ It is described in terms of 3 force components acting across each of 3 mutually orthogonal surfaces
  - ◆ 6 independent parameters
  - ◆ Force  $d\mathbf{F}/dS$  depends on the orientation  $\mathbf{n}$ , but stress *does not*
  - ◆ Stress is best described by a matrix:

$$\begin{pmatrix} dF_x \\ dF_y \\ dF_z \end{pmatrix} = dS \begin{pmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{pmatrix} \begin{pmatrix} n_x \\ n_y \\ n_z \end{pmatrix}$$

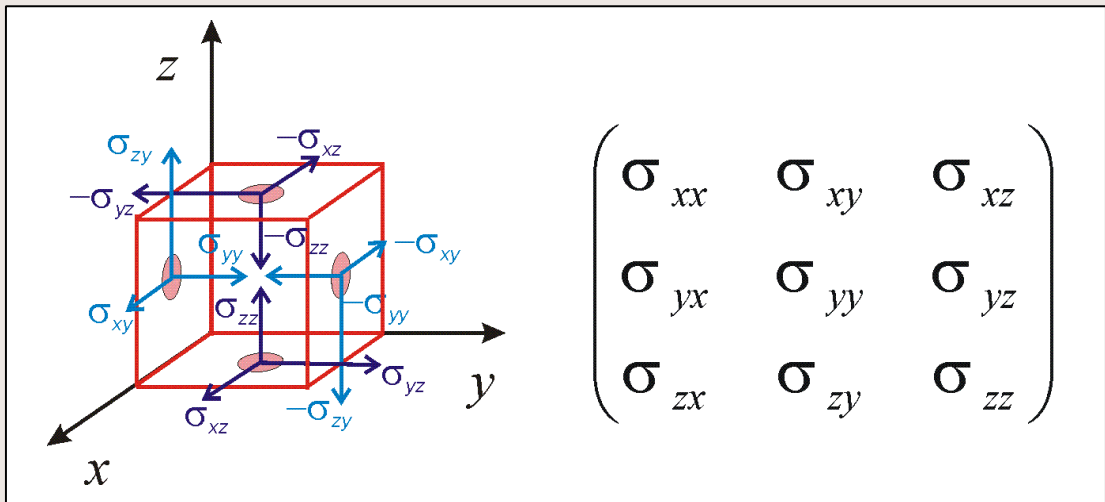
$\sigma_{xy} = \sigma_{yx}$ ,  
 $\sigma_{xz} = \sigma_{zx}$ ,  
 $\sigma_{yz} = \sigma_{zy}$

↑ *Shear stress components are symmetric*
↘ *Normal stress components*

- In a continuous medium, stress depends on  $(x,y,z,t)$  and thus it is a *field*

# Forces acting on a small cube

- Consider a small cube within the elastic body. Assume dimensions of the cube equal '1'
- Both the *forces* and *torque* acting on the cube from the outside are balanced:



- In consequence, the stress tensor is *symmetric*:  $\sigma_{ij} = \sigma_{ji}$
- Just 6 independent parameters out of 9

# Strain

## within a deformed body

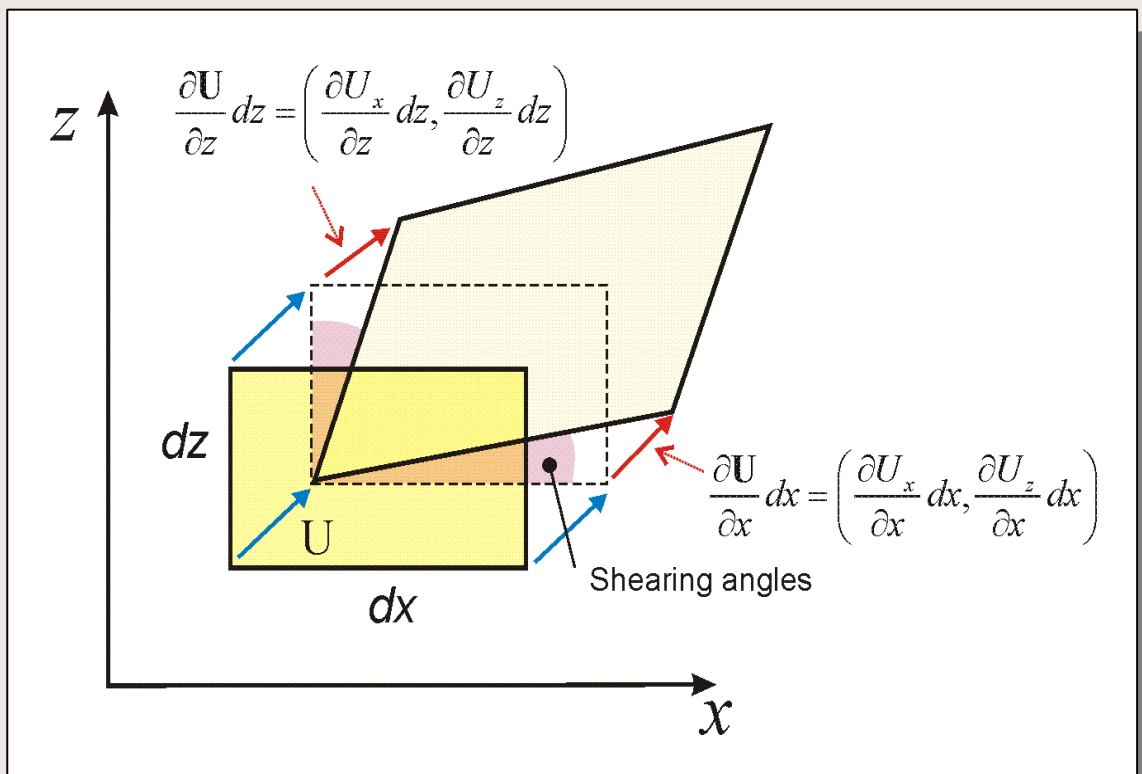
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- Strain is a measure of deformation, *i.e.*, *variation of relative displacement* as associated with a *particular direction* within the body
- It is, therefore, also a *tensor*
  - ♦ Represented by a matrix
  - ♦ Like stress, it is decomposed into *normal* and *shear* components
- Seismic waves yield strains of  $10^{-10}$ - $10^{-6}$ 
  - ♦ So we rely on infinitesimal strain theory



# Elementary Strain

- When a body is deformed, *displacements* ( $\mathbf{U}$ ) of its points are dependent on  $(x,y,z)$ , and consist of:
  - ♦ Translation (**blue arrows** below)
  - ♦ Deformation (**red arrows**)
- Elementary strain is simply 
$$e_{ij} = \frac{\partial U_i}{\partial x_j}$$



# Strain Components

- However, anti-symmetric combinations of  $e_{ij}$  above yield simple rotations of the body without changing its shape:
  - ♦ e.g.,  $\frac{1}{2}\left(\frac{\partial U_z}{\partial x} - \frac{\partial U_x}{\partial z}\right)$  yields rotation about the 'y' axis.
  - ♦ So, the case of  $\frac{\partial U_z}{\partial x} = \frac{\partial U_x}{\partial z}$  is called *pure shear* (no rotation)
- To characterize deformation, only the symmetric component of the elementary strain is used:

$$\epsilon_{ij} = \frac{1}{2} \left( \frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right),$$

$$\epsilon_{ij} = \epsilon_{ji}, \text{ where } i, j = x, y, \text{ or } z$$

$$\epsilon = \begin{pmatrix} \frac{\partial U_x}{\partial x} & \frac{1}{2} \left( \frac{\partial U_x}{\partial y} + \frac{\partial U_y}{\partial x} \right) & \frac{1}{2} \left( \frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) \\ \frac{1}{2} \left( \frac{\partial U_y}{\partial x} + \frac{\partial U_x}{\partial y} \right) & \frac{\partial U_y}{\partial y} & \frac{1}{2} \left( \frac{\partial U_y}{\partial z} + \frac{\partial U_z}{\partial y} \right) \\ \frac{1}{2} \left( \frac{\partial U_z}{\partial x} + \frac{\partial U_x}{\partial z} \right) & \frac{1}{2} \left( \frac{\partial U_z}{\partial y} + \frac{\partial U_y}{\partial z} \right) & \frac{\partial U_z}{\partial z} \end{pmatrix}$$

# Dilatational Strain

(relative volume change during deformation)

- Original volume:  $V = \delta x \delta y \delta z$
- Deformed volume:  $V + \delta V = (1 + \varepsilon_{xx})(1 + \varepsilon_{yy})(1 + \varepsilon_{zz}) \delta x \delta y \delta z$
- Dilatational strain:

$$\Delta = \frac{\delta V}{V} = (1 + \varepsilon_{xx})(1 + \varepsilon_{yy})(1 + \varepsilon_{zz}) - 1 \approx \varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz}$$

$$\Delta = \varepsilon_{ii} = \partial_i U_i = \vec{\nabla} \cdot \vec{U} = \text{div } \vec{U}$$

- Note that (as expected) shearing strain does not change the volume

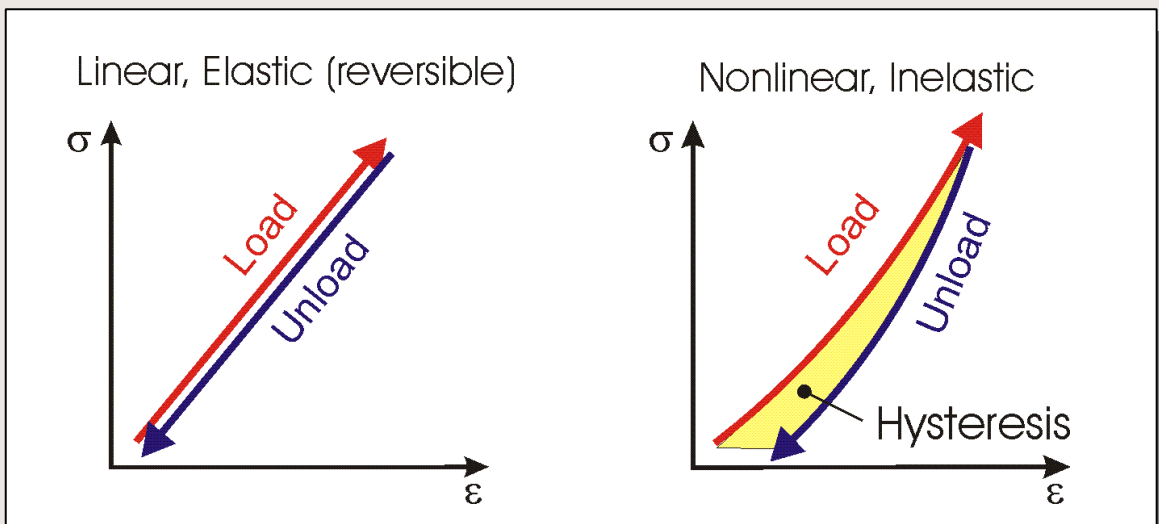


# Hooke's Law (general)

- Describes the **stress** developed in a **deformed body**:

$\mathbf{F} = -k\mathbf{x}$  for an ordinary spring (1-D)

$\sigma \sim \varepsilon$  (in some sense) for a '*linear*', '*elastic*'  
3-D solid. This is what it means:



- For a general (*anisotropic*) medium, there are 36 coefficients of proportionality between six independent  $\sigma_{ij}$  and six  $\varepsilon_{ij}$ .

# Hooke's Law (isotropic medium)

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• For isotropic medium, the strain/stress relation is described by just two constants:

$$\sigma_{ij} = \lambda\Delta + 2\mu\varepsilon_{ij} \text{ for normal strain/stress}$$

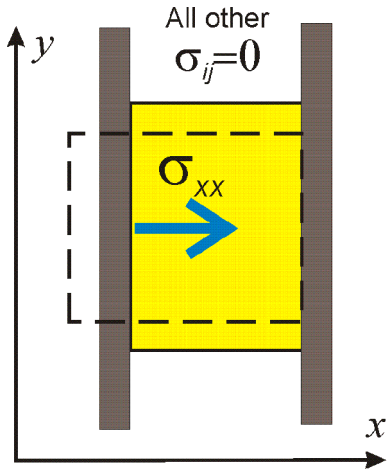
( $i=j$ , where  $i,j = x,y,z$ )

$$\sigma_{ij} = 2\mu\varepsilon_{ij} \text{ for shear components } (i \neq j)$$

- ♦  $\lambda$  and  $\mu$  are called the *Lamé constants*.

# Four Elastic Moduli

- Depending on boundary conditions (*i.e.*, experimental setup) different combinations of  $\lambda$  and  $\mu$  may be convenient. These combinations are called *elastic constants*, or *moduli*:
  - **Young's modulus and Poisson's ratio:**
    - ♦ Consider a cylindrical sample uniformly squeezed along axis  $X$ :



All other  $\sigma_{ij} = 0$

$$\sigma_{xx} = \lambda' \Delta + 2\mu \varepsilon_{xx}$$

$$\sigma_{yy} = \lambda' \Delta + 2\mu \varepsilon_{yy} = 0,$$

$$\sigma_{zz} = \lambda' \Delta + 2\mu \varepsilon_{zz} = 0 \Rightarrow \varepsilon_{yy} = \varepsilon_{zz} = \frac{-\lambda' \Delta}{2\mu}.$$

Young's modulus:  $E = \frac{\sigma_{xx}}{\varepsilon_{xx}} = \frac{2\mu(3\lambda' + 2\mu)}{\lambda' + \mu}$

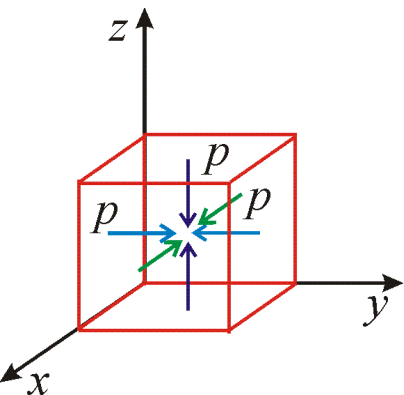
Poisson's ratio:  $\nu = -\frac{\varepsilon_{zz}}{\varepsilon_{xx}} = \frac{\lambda'}{2(\lambda' + \mu)}$

Often denoted  $\sigma$ .

# Four Elastic Moduli

- Bulk modulus,  $K$

- ◆ Consider a cube subjected to hydrostatic pressure



$$\sigma_{xx} = \sigma_{yy} = \sigma_{zz} = -p,$$

$$-3p = 3\lambda'\Delta + 2\mu\Delta$$

Bulk modulus:  $K = \frac{-p}{\Delta} = \lambda' + \frac{2}{3}\mu$

- Finally, the constant  $\mu$  complements  $K$  in describing the shear rigidity of the medium, and thus it is also called '*rigidity modulus*'

- For rocks:

- ◆ Generally,  $10 \text{ Gpa} < \mu < K < E < 200 \text{ Gpa}$
  - ◆  $0 < \nu < \frac{1}{2}$  always; for rocks,  $0.05 < \nu < 0.45$ , for most “hard rocks”,  $\nu$  is near 0.25.

- For fluids,  $\nu = \frac{1}{2}$  and  $\mu = 0$  (no shear resistance)

# Strain/Stress Energy Density

- Mechanical work is required to deform an elastic body; as a result, elastic energy is accumulated in the strain/stress field
- When released, this energy gives rise to earthquakes and seismic waves
- For a loaded spring (1-D elastic body),  
 $E = \frac{1}{2}kx^2 = \frac{1}{2}Fx$
- Similarly, for a deformed elastic medium, *energy density* is:

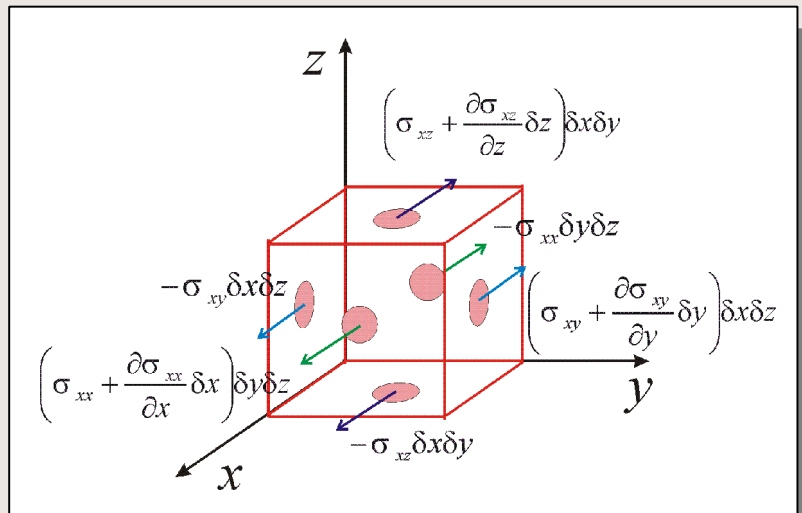
$$E = \frac{1}{2} \sum_{i,j=x,y,z} \sigma_{ij} \epsilon_{ij}$$

- Energy density (per unit volume) is thus measured in:  $\left[ \frac{\text{Newton} \cdot \text{m}}{\text{m}^3} \right] = \left[ \frac{\text{Newton}}{\text{m}^2} \right]$



# Inhomogeneous Stress

If stress is inhomogeneous (variable in space), its derivatives result in a *net force* acting on an infinitesimal volume:



$$\begin{aligned}
 F_x = & \left[ \left( \sigma_{xx} + \frac{\partial \sigma_{xx}}{\partial x} \delta x \right) - \sigma_{xx} \right] \delta y \delta z \\
 & + \left[ \left( \sigma_{xy} + \frac{\partial \sigma_{xy}}{\partial y} \delta y \right) - \sigma_{xy} \right] \delta x \delta z \\
 & + \left[ \left( \sigma_{xz} + \frac{\partial \sigma_{xz}}{\partial z} \delta z \right) - \sigma_{xz} \right] \delta x \delta y = \left( \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + \frac{\partial \sigma_{xz}}{\partial z} \right) \delta y \delta x \delta z
 \end{aligned}$$

Thus, for  $i = x, y, z$ :

$$F_i = \left( \frac{\partial \sigma_{ix}}{\partial x} + \frac{\partial \sigma_{iy}}{\partial y} + \frac{\partial \sigma_{iz}}{\partial z} \right) \delta V$$

# Equations of Motion

(Govern motion of the elastic body with time)

Uncompensated net force will result in *acceleration* (Newton's law):

Newton's law: 
$$\rho \delta V \frac{\partial^2 U_i}{\partial t^2} = F_i$$

$$\rho \frac{\partial^2 U_i}{\partial t^2} = \left( \frac{\partial \sigma_{ix}}{\partial x} + \frac{\partial \sigma_{iy}}{\partial y} + \frac{\partial \sigma_{iz}}{\partial z} \right)$$

$$\begin{aligned} \rho \frac{\partial^2 U_x}{\partial t^2} &= \frac{\partial}{\partial x} \left( \lambda' \Delta + 2\mu \frac{\partial U_x}{\partial x} \right) + \mu \frac{\partial}{\partial y} \left( \frac{\partial U_x}{\partial y} + \frac{\partial U_y}{\partial x} \right) + \mu \frac{\partial}{\partial z} \left( \frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) \\ &= \lambda' \frac{\partial \Delta}{\partial x} + \mu \frac{\partial}{\partial x} \left( \frac{\partial U_x}{\partial x} + \frac{\partial U_y}{\partial y} + \frac{\partial U_z}{\partial z} \right) + \mu \left( \frac{\partial^2 U_x}{\partial x^2} + \frac{\partial^2 U_x}{\partial y^2} + \frac{\partial^2 U_x}{\partial z^2} \right) \\ &= (\lambda' + \mu) \frac{\partial \Delta}{\partial x} + \mu \nabla^2 U_x \end{aligned}$$

These are the *equations of motion* for each of the components of **U**:

$$\begin{aligned} \rho \frac{\partial^2 U_x}{\partial t^2} &= (\lambda' + \mu) \frac{\partial \Delta}{\partial x} + \mu \nabla^2 U_x \\ \rho \frac{\partial^2 U_y}{\partial t^2} &= (\lambda' + \mu) \frac{\partial \Delta}{\partial y} + \mu \nabla^2 U_y \\ \rho \frac{\partial^2 U_z}{\partial t^2} &= (\lambda' + \mu) \frac{\partial \Delta}{\partial z} + \mu \nabla^2 U_z \end{aligned}$$

# Wave Equation

(Propagation of  
compressional/acoustic waves)

- To show that these three equations describe several types of waves, first let's apply *divergence operation* to them:

$$\rho \frac{\partial^2 \Delta}{\partial t^2} = (\lambda' + \mu) \nabla^2 \Delta + \mu \nabla^2 \Delta = (\lambda' + 2\mu) \nabla^2 \Delta$$

- This *is* a wave equation; compare to the general form of equation describing wave processes:

$$\left[ \frac{1}{c^2} \frac{\partial^2}{\partial t^2} - \nabla^2 \right] f(x, y, z, t) = 0$$

- Above, *c* is the wave velocity.

- We have:

$$\left[ \frac{\rho}{(\lambda' + 2\mu)} \frac{\partial^2}{\partial t^2} - \nabla^2 \right] \Delta = 0$$

- This equation describes *compressional (P)* waves

- P*-wave velocity:

$$v_p = \sqrt{\frac{\lambda + 2\mu}{\rho}} = \sqrt{\frac{K + \frac{4}{3}\mu}{\rho}}$$

# Wave Equation

## (Propagation of shear waves)

- Similarly, let's apply the *curl operation* to the equations for  $\mathbf{U}$  (remember,  $\mathbf{curl}(\mathbf{grad}) = 0$  for any field:

$$\rho \frac{\partial^2}{\partial t^2} \mathbf{curl} \mathbf{U} = \mu \nabla^2 \mathbf{curl} \mathbf{U}$$

- This is also a wave equation; again compare to the general form:

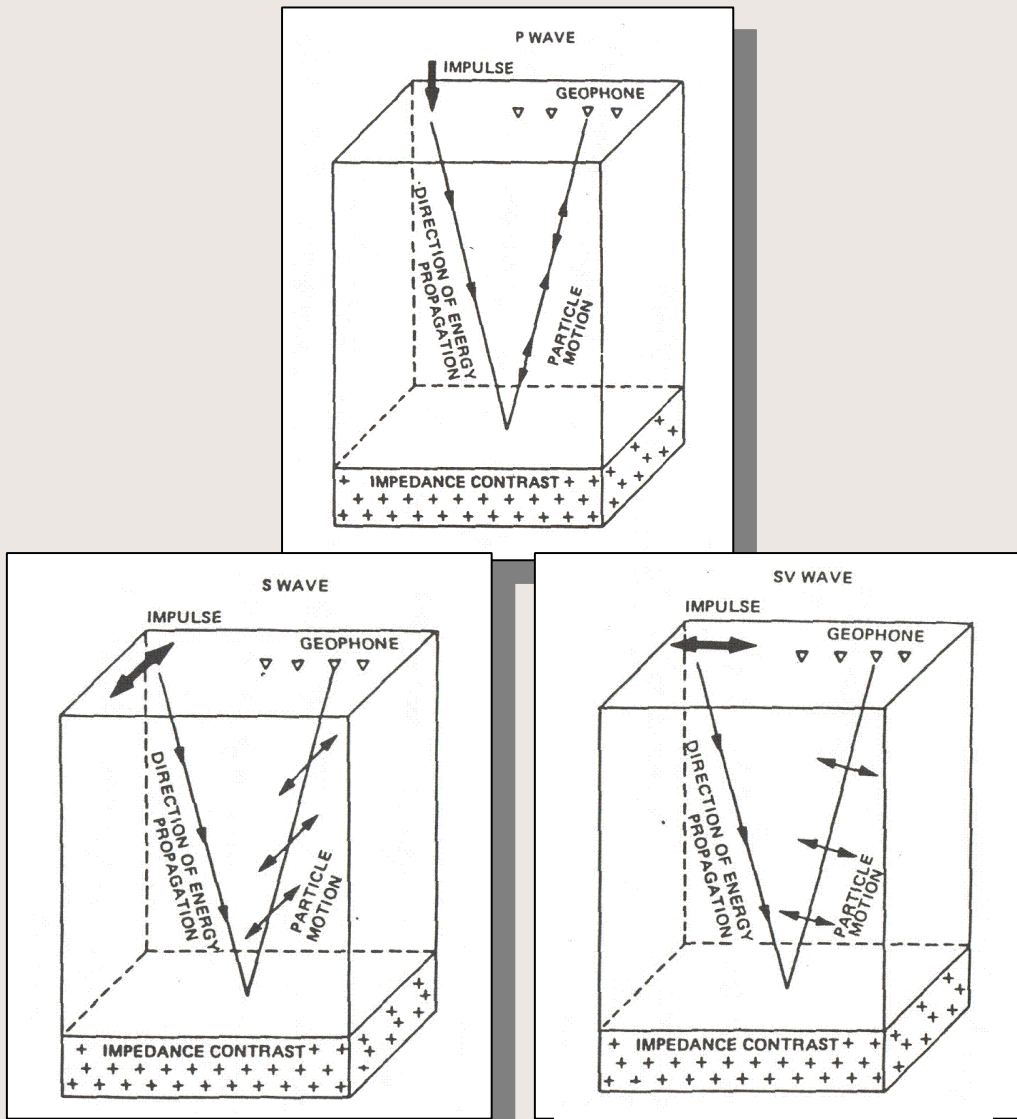
$$\left[ \frac{1}{c^2} \frac{\partial^2}{\partial t^2} - \nabla^2 \right] f(x, y, z, t) = 0$$

- This equation describes *shear* ( $S$ ), or *transverse* waves.
- Since it involves rotation, there is no associated volume change, and particle motion is *across* the wave propagation direction.
- Its velocity:  $V_S < V_P$ ,
- For  $\nu = 0.25$ ,  $V_P/V_S = \sqrt{3}$

$$V_S = \sqrt{\frac{\mu}{\rho}}$$

# Wave Polarization

- Thus, elastic solid supports two types of *body waves*:



$$v_p = \sqrt{\frac{\lambda + 2\mu}{\rho}} = \sqrt{\frac{K + \frac{4}{3}\mu}{\rho}}, \quad v_s = \sqrt{\frac{\mu}{\rho}}$$



# Waveforms and wave fronts

## Plane waves

- Consider the wave equation:

$$\left[ \frac{1}{c^2} \frac{\partial^2}{\partial t^2} - \nabla^2 \right] f(x, y, z, t) = 0$$

- Why does it describe a wave? Note that it is satisfied with any function of the form:

$$f(x, y, z, t) = \varphi(x - ct)$$

$$f(x, y, z, t) = \varphi(x + ct)$$

- The function  $\varphi()$  is *the waveform*. Note that the entire waveform propagates with time to the right or left along the x-axis,  $x = \pm ct$ . This is what is called the *wave process*.
- The argument of  $\varphi(\dots)$  is called *phase*.
- Surfaces of constant phase are called *wavefronts*.
  - ♦ In our case, the wavefronts are planes:
 
$$x = \text{phase} \pm ct$$
 for any  $(y, z)$ . For this reason, the above solutions are *plane waves*.

# Waveforms and wave fronts

## Non-planar waves

- The wave equation is also satisfied by such solutions (*spherical waves*):

$$f(\mathbf{x}, t) = \frac{1}{|\mathbf{r}|} \phi(|\mathbf{r}| - Vt)$$

$$f(\mathbf{x}, t) = \frac{1}{|\mathbf{r}|} \phi(|\mathbf{r}| + Vt)$$

...and by such (*cylindrical waves*):

$$f(\mathbf{x}, t) = \frac{1}{\sqrt{\rho}} \phi(\rho - Vt)$$

$$f(\mathbf{x}, t) = \frac{1}{\sqrt{\rho}} \phi(\rho + Vt)$$

...and by various others.

**Question:** what is the problem with the second solution in each pair?

# Waves and sources

- Homogeneous wave equation describes **free waves**:

$$\left( \frac{1}{c^2} \frac{\partial^2}{\partial t^2} - \nabla^2 \right) f = 0$$

- plane, spherical, cylindrical...
- incoming, outgoing...

- Inhomogeneous equation describes waves generated by a **source**:

$$\left( \frac{1}{c^2} \frac{\partial^2}{\partial t^2} - \nabla^2 \right) f = source$$

- Note that this also includes all of the free waves, and so one also needs **boundary conditions** to specify a unique solution
  - For example, no waves come from infinity toward the source (“*radiation condition*”)